

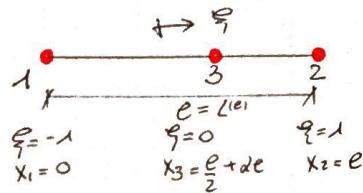
ASSIGNMENT 4

ASSIGNMENT 4.1.

3-node straight bar element. length $\ell = x_1 - x_2$. Isoparametric definition:

$$\begin{bmatrix} 1 \\ x \\ u \end{bmatrix} = \begin{bmatrix} 1 & 1 & 1 \\ x_1 & x_2 & x_3 \\ u_1 & u_2 & u_3 \end{bmatrix} \begin{bmatrix} N_{1e} \\ N_{2e} \\ N_{3e} \end{bmatrix} \quad N_i^e(\xi) = \text{shape functions.}$$

$$\begin{cases} x_1 = 0 \\ x_2 = \ell \\ x_3 = \left(\frac{1}{2} + \alpha\right)\ell \end{cases} \quad -\frac{1}{2} < \alpha < \frac{1}{2}$$



- (i) Get Jacobian $J = \frac{dx}{d\xi}$. Show that :
- if $-\frac{1}{2} < \alpha < \frac{1}{2}$ and $J > 0$ over the whole element
 $-1 \leq \xi \leq 1$.
 - if $\alpha = 0$, $J = \frac{1}{2}$ is a constant over the element.

We obtain the shape functions by using Lagrange polynomials.

- $N_1 = \frac{(\xi-0)(\xi-1)}{(-1)(-1-1)} = \frac{1}{2}\xi(\xi-1)$
- $N_2 = \frac{(\xi+1)(\xi+0)}{(1+1)(1+0)} = \frac{1}{2}\xi(\xi+1) \Rightarrow X = \sum N_i x_i = N_1 x_1 + N_2 x_2 + N_3 x_3$
- $N_3 = \frac{(\xi+1)(\xi-1)}{(1)(-1)} = 1 - \xi^2 = \frac{\xi e}{2}(\xi+1) + (1-\xi^2)\left[\frac{\ell}{2} + \alpha\ell\right]$

So the Jacobian will be:

$$\frac{dx}{d\xi} = \frac{\ell}{2} (2\xi + 1) - 2\xi\left(\frac{\ell}{2} + \alpha\ell\right)$$

- if $-\frac{1}{2} < \alpha < \frac{1}{2}$ and $J > 0 \rightarrow \boxed{\alpha = -\frac{1}{4}}$ for whole element $J > 0$ as show.

$$J = \frac{1}{2} (2\eta + 1) - 2\eta \left(\frac{\ell}{2} - \frac{\ell}{4}\right) = \frac{\ell}{2} [2\eta + 1 - 2\eta] = \frac{\ell}{2}$$

So: $J = \frac{\ell}{2} - \frac{2\eta}{\ell} x > 0$

- if $\alpha = 0 \rightarrow J = \frac{\ell}{2}$ constant!

(ii) Obtain the 1×3 strain displacement matrix B relating $e = \frac{du}{dx} = Bu^e$
where u^e is the column 3-vector of the node displacement
 u_1, u_2 and u_3 . The entries of B are functions of $1, \alpha, \xi$.

Hint: $B = \frac{dN}{dx} = J^{-1} \frac{dN}{d\eta}$ where $\begin{cases} N = [N_1 \ N_2 \ N_3] \\ J = \text{Jacobian} \end{cases}$

$$B = \frac{1}{\ell \left(\frac{1}{2} - 2\eta \alpha \right)} \begin{bmatrix} \frac{1}{2} (2\eta - 1) & \frac{1}{2} (2\eta + 1) & -2\eta \end{bmatrix}$$

$$= J^{-1} \begin{bmatrix} \frac{dN_1}{d\eta} & \frac{dN_2}{d\eta} & \frac{dN_3}{d\eta} \end{bmatrix}.$$

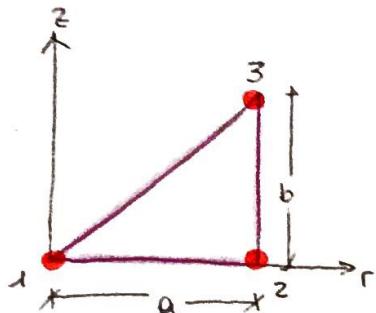
ASSIGNMENT 4.2.

(a) Compute the entries of \mathbf{K}^e for the following axisymmetric triangle:

$$r_1 = 0 \quad r_2 = r_3 = a \quad z_1 = z_2 = 0 \quad z_3 = b$$

The material is isotropic with $J=0$ for which the stress-strain matrix is:

$$\mathbf{E} = E \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 1/2 \end{bmatrix}$$



By the isoparametric formulation we have:

$$\begin{bmatrix} 1 \\ r \\ z \\ u_r \\ u_z \end{bmatrix} = \begin{bmatrix} 1 & 1 & 1 \\ r_1 & r_2 & r_3 \\ z_1 & z_2 & z_3 \\ u_{r1} & u_{r2} & u_{r3} \\ u_{z1} & u_{z2} & u_{z3} \end{bmatrix} \begin{bmatrix} N_1 \\ N_2 \\ N_3 \end{bmatrix} = \begin{bmatrix} 1 & 1 & 1 \\ 0 & a & a \\ 0 & 0 & b \\ u_{r1} & u_{r2} & u_{r3} \\ u_{z1} & u_{z2} & u_{z3} \end{bmatrix} \begin{bmatrix} \varphi \\ u \\ v \end{bmatrix}$$

φ, u, v = natural coordinates.

We need a coordinate transformation because the shape functions are given on natural coordinates.

$$\begin{bmatrix} N_1 \\ N_2 \\ N_3 \end{bmatrix} = \begin{bmatrix} 1 & 1 & 1 \\ 0 & a & a \\ 0 & 0 & b \end{bmatrix}^{-1} \begin{bmatrix} 1 \\ r \\ z \end{bmatrix}$$

$$\begin{bmatrix} dN_i/dr \\ dN_i/dz \end{bmatrix} = J^{-1} \begin{bmatrix} \partial N_i / \partial \varphi \\ \partial N_i / \partial u \end{bmatrix}$$

$$\text{where } J = \text{Jacobian} = \begin{bmatrix} \frac{\partial r}{\partial \varphi} & \frac{\partial z}{\partial \varphi} \\ \frac{\partial r}{\partial u} & \frac{\partial z}{\partial u} \end{bmatrix}$$

$$J = \begin{bmatrix} -a & -b \\ 0 & -b \end{bmatrix}$$

- Calculate the derivative of the shape functions: (natural coordinates)

$$\begin{bmatrix} \partial N_1 / \partial \varphi \\ \partial N_1 / \partial u \end{bmatrix} = \begin{bmatrix} 1 \\ 0 \end{bmatrix} \quad \begin{bmatrix} \partial N_2 / \partial \varphi \\ \partial N_2 / \partial u \end{bmatrix} = \begin{bmatrix} 0 \\ 1 \end{bmatrix} \quad \begin{bmatrix} \partial N_3 / \partial \varphi \\ \partial N_3 / \partial u \end{bmatrix} = \begin{bmatrix} -1 \\ -1 \end{bmatrix}$$

so the vector N is defined below:

$$N = \begin{bmatrix} N_1 & 0 & N_2 & 0 & N_3 & 0 \\ 0 & N_1 & 0 & N_2 & 0 & N_3 \end{bmatrix}$$

Global coordinates:

$$\begin{cases} N_1 = 1 - \frac{r}{a} \\ N_2 = \frac{r}{a} - \frac{z}{b} \\ N_3 = \frac{z}{b} \end{cases}$$

We know that the matrix B is defined by:

$$B = D \times N$$

And so:

$$B = \begin{bmatrix} -1/a & 0 & 1/a & 0 & 0 & 0 \\ 0 & 0 & 0 & -1/b & 0 & 1/b \\ 1/r - 1/a & 0 & 1/a - z/b & 0 & z/b & 0 \\ 0 & -1/a & -1/b & 1/a & 1/b & 0 \end{bmatrix}$$

And the STIFFNESS-MATRIX $K^e = \int_{\Omega} 2\pi r B^T E B dA$

for the axisymmetry we can reduce the volume integral of the 3D triangle into an area integral \times the circumference.

↓

$$2\pi \int_{\Omega} r (1/z) dA$$

So we have:

$$K^e = 2\pi \int_{\Omega} r B^T E B dA = 2\pi \frac{1}{2} \int_{-1}^1 \int_{-1}^1 r B^T E B \det J d\eta d\eta$$

↑ Jacobian matrix
↑ natural coordinate

To integrate the stiffness matrix we use the one-point Gauss integration in which the evaluated function must be integrated on the middle of the natural domain (zero).

$$K^e = E \begin{bmatrix} 5b/12 & 0 & -b/4 & 0 & b/12 & 0 \\ 0 & b/6 & a/6 & -b/6 & -a/6 & 0 \\ -b/4 & a/6 & 5b/12 + a^2/6b & -a/6 & b/12 - a^2/6b & 0 \\ 0 & -b/6 & -a/6 & b/6 + a^2/3b & a/6 & -a^2/3b \\ b/12 & -a/6 & b/12 a^2/6b & a/6 & b/12 + a^2/6b & 0 \\ 0 & 0 & 0 & -a^2/3b & 0 & a^2/3b \end{bmatrix}$$

- (2) Show that the sum of the rows (1 columns) 2,4,6 of K^e must be vanish and explain why.
 Show as well that the sum of rows (1 columns) 1,3,5 does not vanish, and explain why.

Sum of rows 2,4,6:

$$\begin{aligned}
 & [b/6 + a/6 + (-b/6) + (-a/6)] + \text{row 2} \\
 & + [-b/6 - a/6 + b/6 + a^2/3b + a/6 - a^2/3b] + \text{row 4} \\
 & + [-a^2/3b + a^2/3b] = \text{row 6} \\
 & = 0! \leftarrow \text{This is because the element could have a rigid motion inside the model.}
 \end{aligned}$$

The same for the columns 2,4,6.

Sum of rows 1,3,5:

$$\begin{aligned}
 & [5b/12 + (-b/4) + b/12] + \\
 & + [-b/4 + a/6 + [b/12 + a^2/6b] - a/6 + b/12 - a^2/6b] + \\
 & + [b/12 - a/6 + b/12 - a^2/6b + a/6 + b/12 + a^2/6b] \neq 0.
 \end{aligned}$$

This because the equations inside rows 1,3,5 ensure that the symmetry will be not violated, which would occur if there is a movement of the rigid body.

(3) Compute the consistent force vector f^e for gravity forces

$$b = [0, -g]^T.$$

As the stiffness matrix, we can calculate the consistent force vector by using one-point Gauss integration.

$$f^e = \frac{1}{2} \sum_{k=1}^{\ell} \sum_{l=1}^p w_k w_l N_{k,l}^T b_{k,l} \det J_{k,l}$$

and we obtain:

$$f^e = \begin{bmatrix} 0 \\ -a^2 bg/9 \\ 0 \\ -a^2 bg/9 \\ 0 \\ -a^2 bg/9 \end{bmatrix}$$