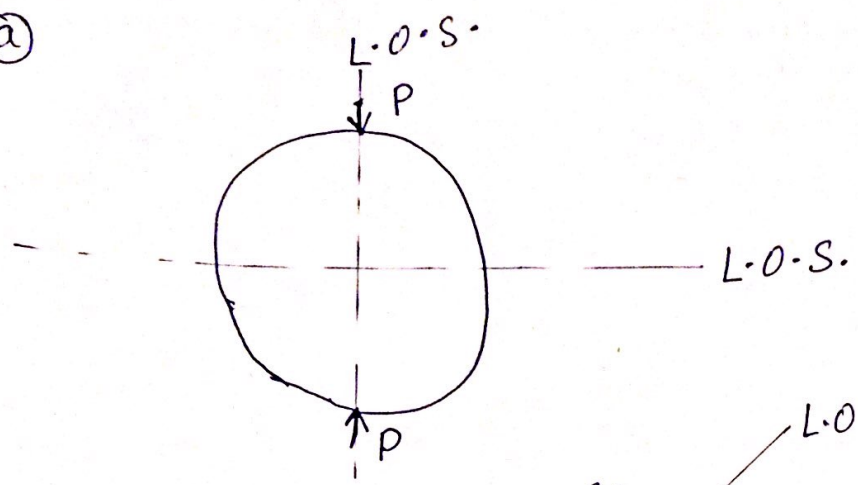


①

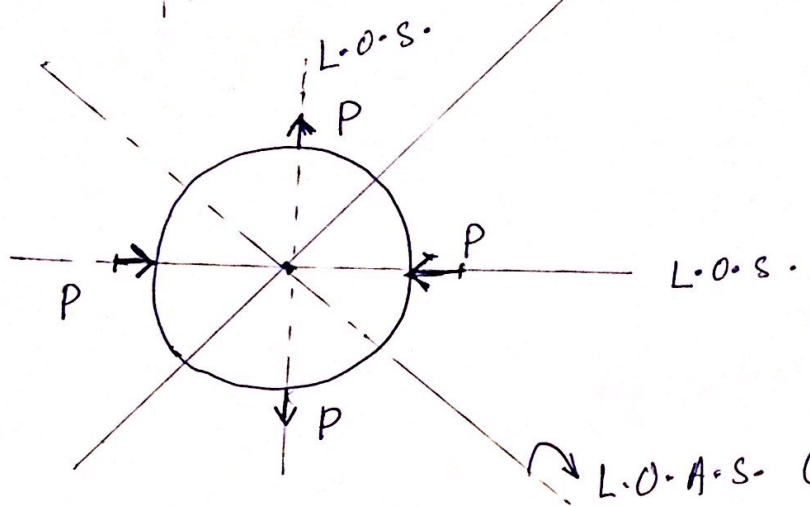
L.O.S. → Line of Symmetry
L.O.A.S. → Line of Antisymmetry

①



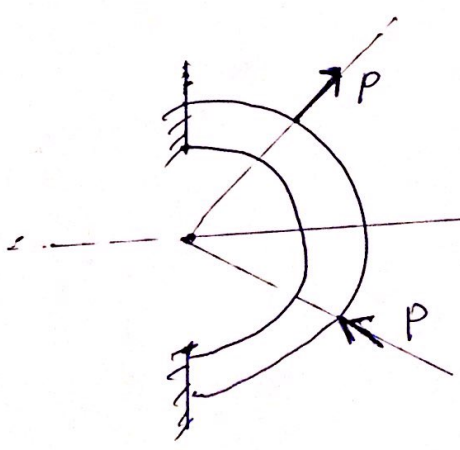
L.O.A.S. (Anti-symmetry)

②



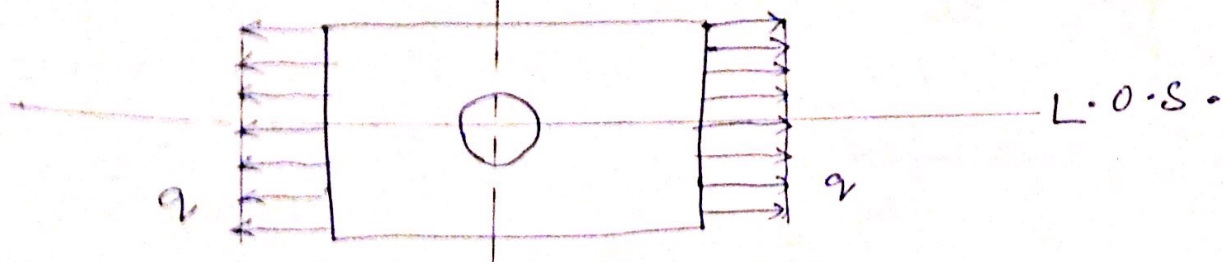
L.O.A.S. (Anti-symmetry)

③

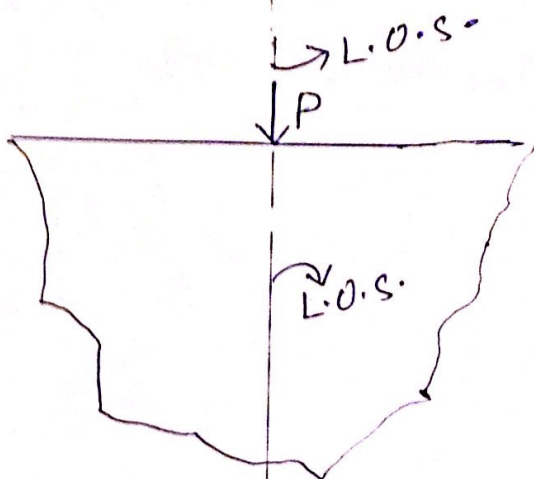


L.O.A.S. (Anti-symmetry)

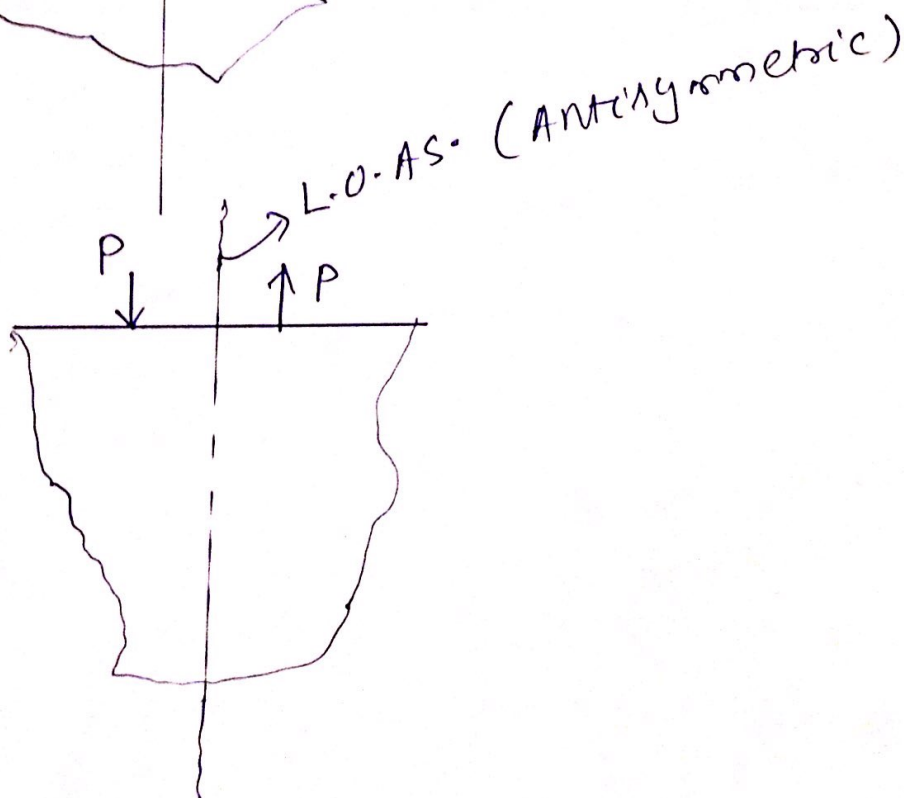
(1)



(2)

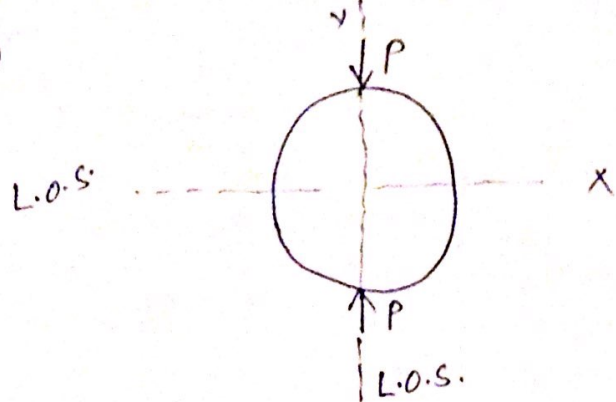


(3)



2 (a)

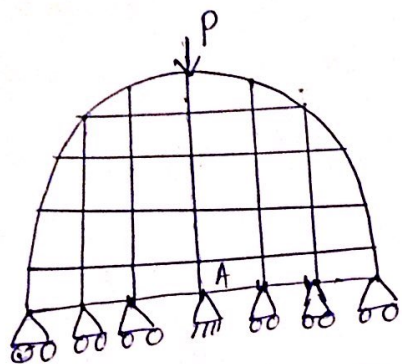
a-1 3



It is possible to cut the complete structure to both one half and one quarter.

ONE HALF :-

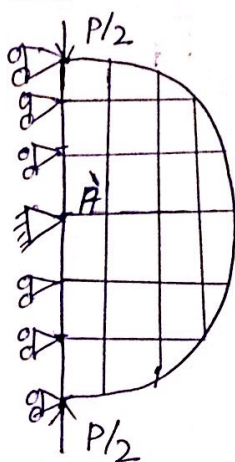
(a) It can be cut along x axis



→ No displacement in the y direction for any point on the x axis.

→ To preclude horizontal rigid body motion node 'A' is constrained in the x, y directions.

(b) It can be cut along y axis



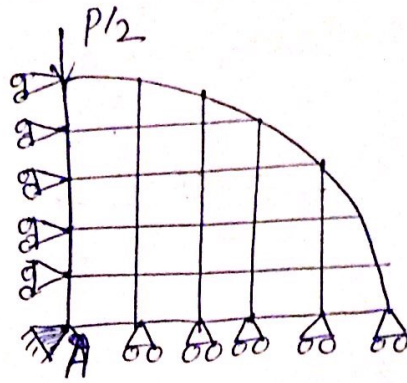
→ No displacement in the x direction for any point on the y axis.

→ To preclude vertical rigid body motion node 'A' is constrained in the x, y directions.

(A' is constrained both in x, y directions)

2.(a)

one quarter:-

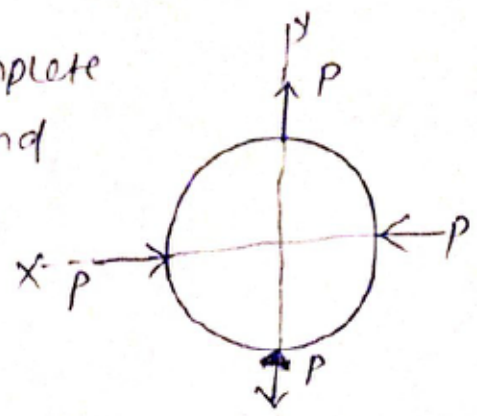


267
vite

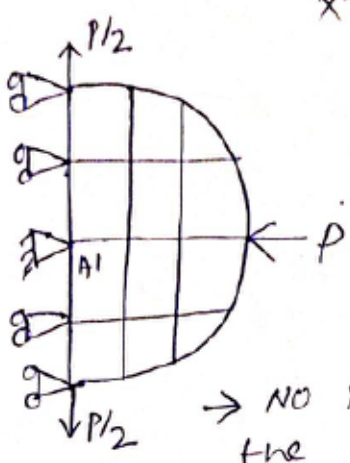
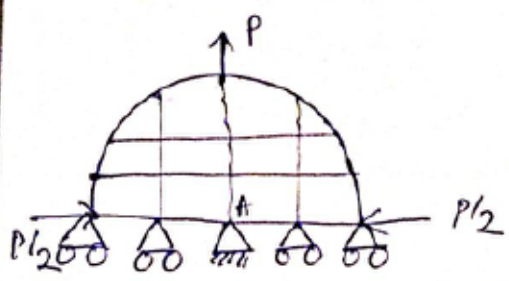
As the structure is doubly symmetric in both geometry and loading, it is evident that no y displacements are possible for points on the x axis and no x displacements are possible for any points on the y axis.

(b)

It is possible to cut the complete structure to both one half and one quarter.



one half

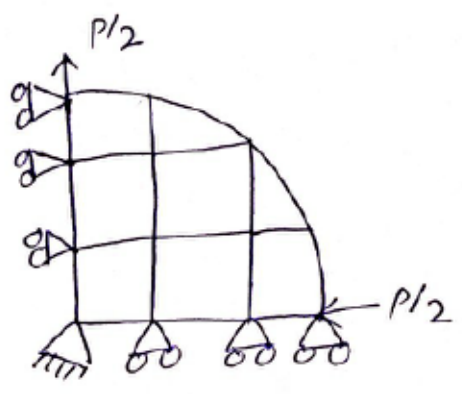


- No y displacements of the points on the x-axis.
- To preclude horizontal rigid body motion node 'A' is constrained in the x direction.

- No x displacement of the points on the y axis
- To preclude vertical rigid body motion node 'A' is constrained in the y direction.

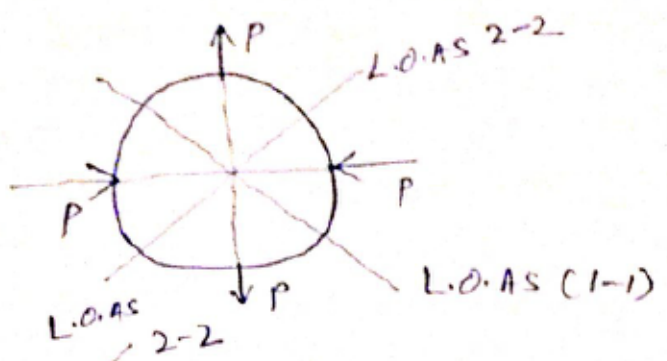
one fourth

→ As the structure is doubly symmetric in both geometry and loading, it is evident that no y displacements are possible for points on the x axis and no x displacements are possible for any points on the y axis.

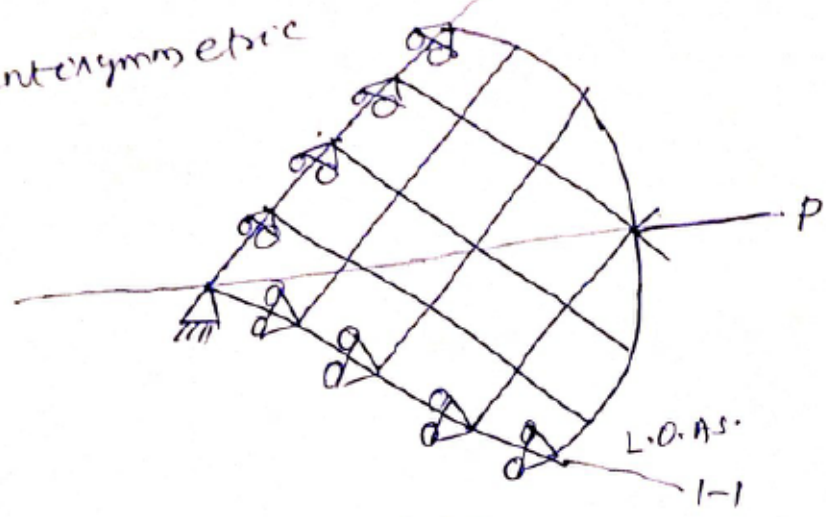


It also can be cut to both one half and one quarter along the line of Anti-symmetries.

one quarter



(1-1) are antisymmetric
2-2 Lines



The displacement of the nodes which are on 1-1 are zero along 1-1; the displacement of the nodes which are on 2-2 are zero along 2-2.

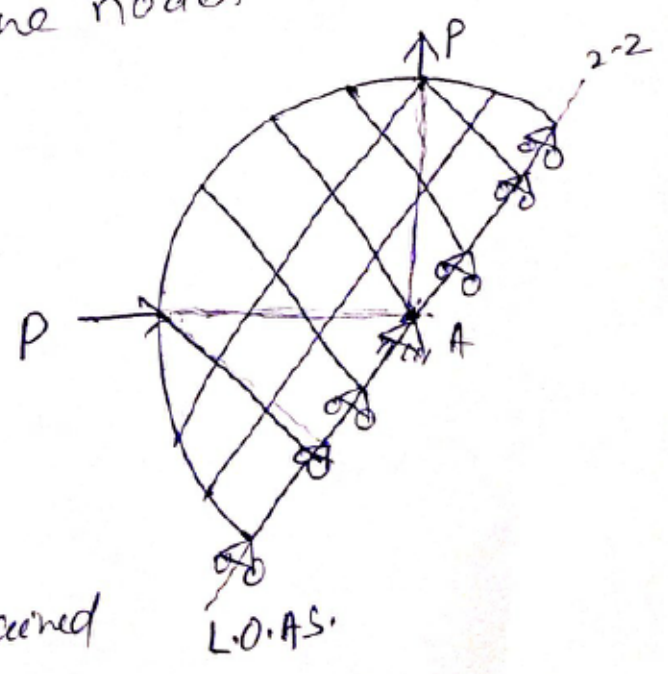
one half

The displacement of nodes on 2-2 are zero along 2-2.

The node A is constrained

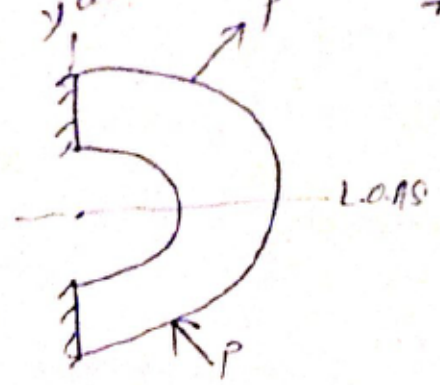
in both 1-1 and 2-2

to prevent rigid body motion.

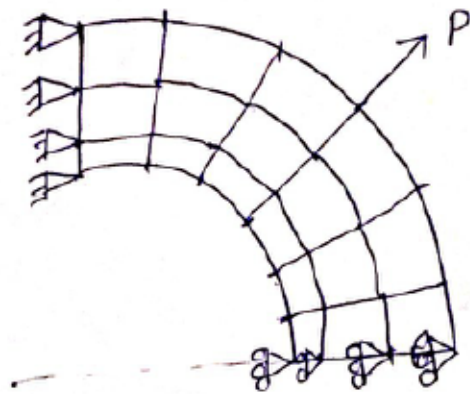


2. ©

It is possible to cut the structure to one half along the Line of Antisymmetry. (x axis)



one half

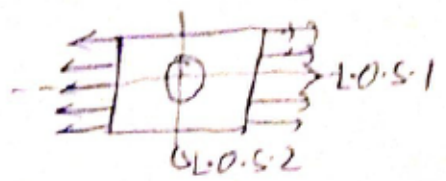


(L.O.S Antisymmetry)

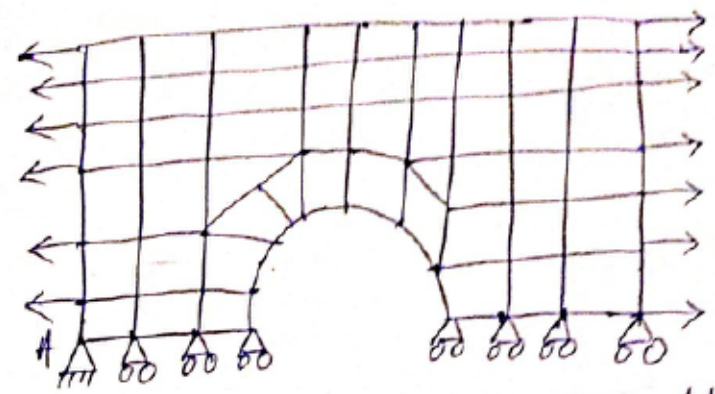
- NO displacements in both the x and y axes for the points on the y axis. (fixed position)
- NO x displacements for the points on the Line of Antisymmetry (x axis) of the structure.

It is possible to cut the complete structure to both one half and one quarter.

ONE HALF :-



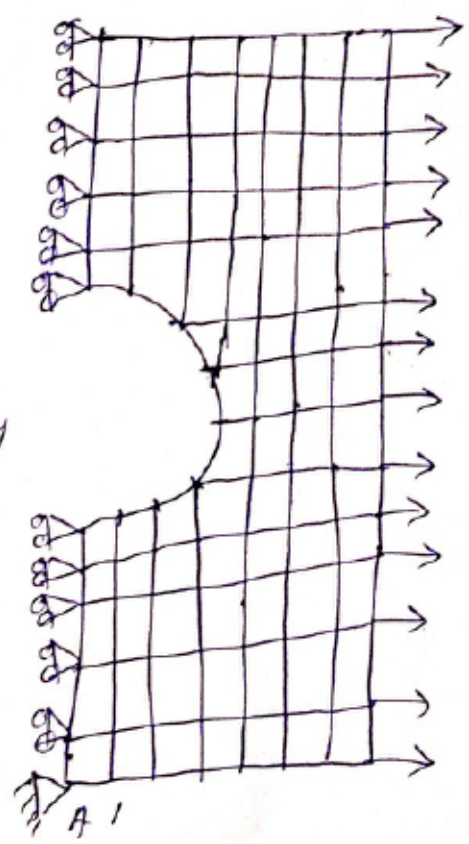
Along L.O.S:1



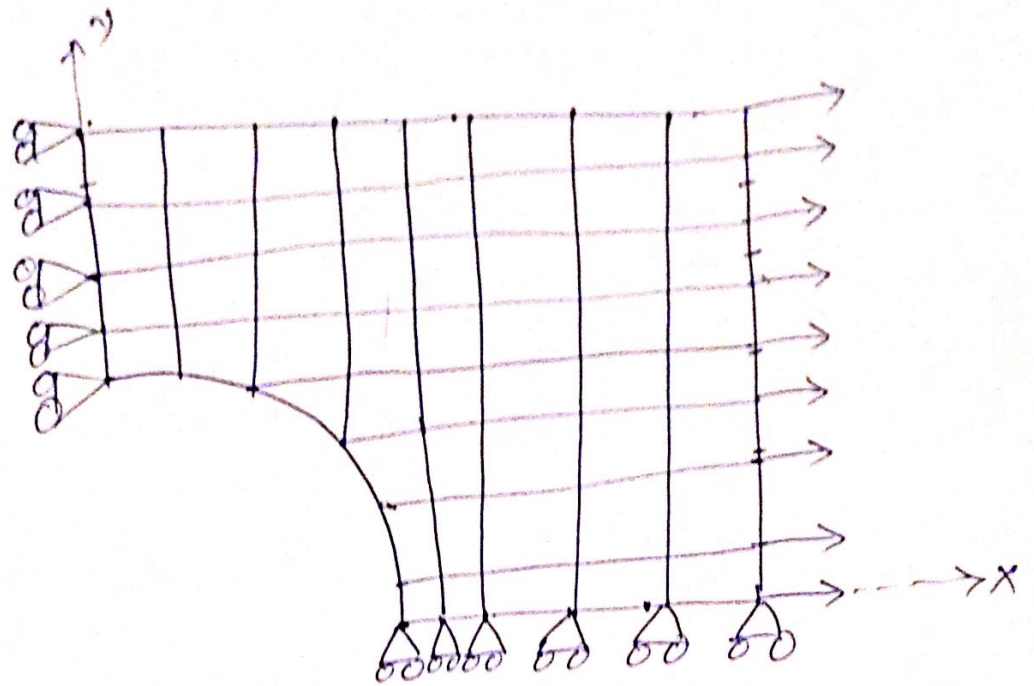
The y displacements of the nodes of the structure on the x axis are zero. To prevent rigid body motion in the x direction, both the x, y displacements of the node A are constrained.

Along L.O.S:2

→ The x displacement of the nodes of the structure on the y axis are zero.
 → To preclude rigid body motion in the y direction, both the x and y displacements of the node A' are constrained.



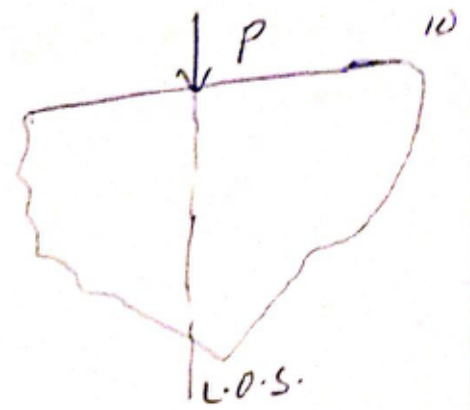
one quarter



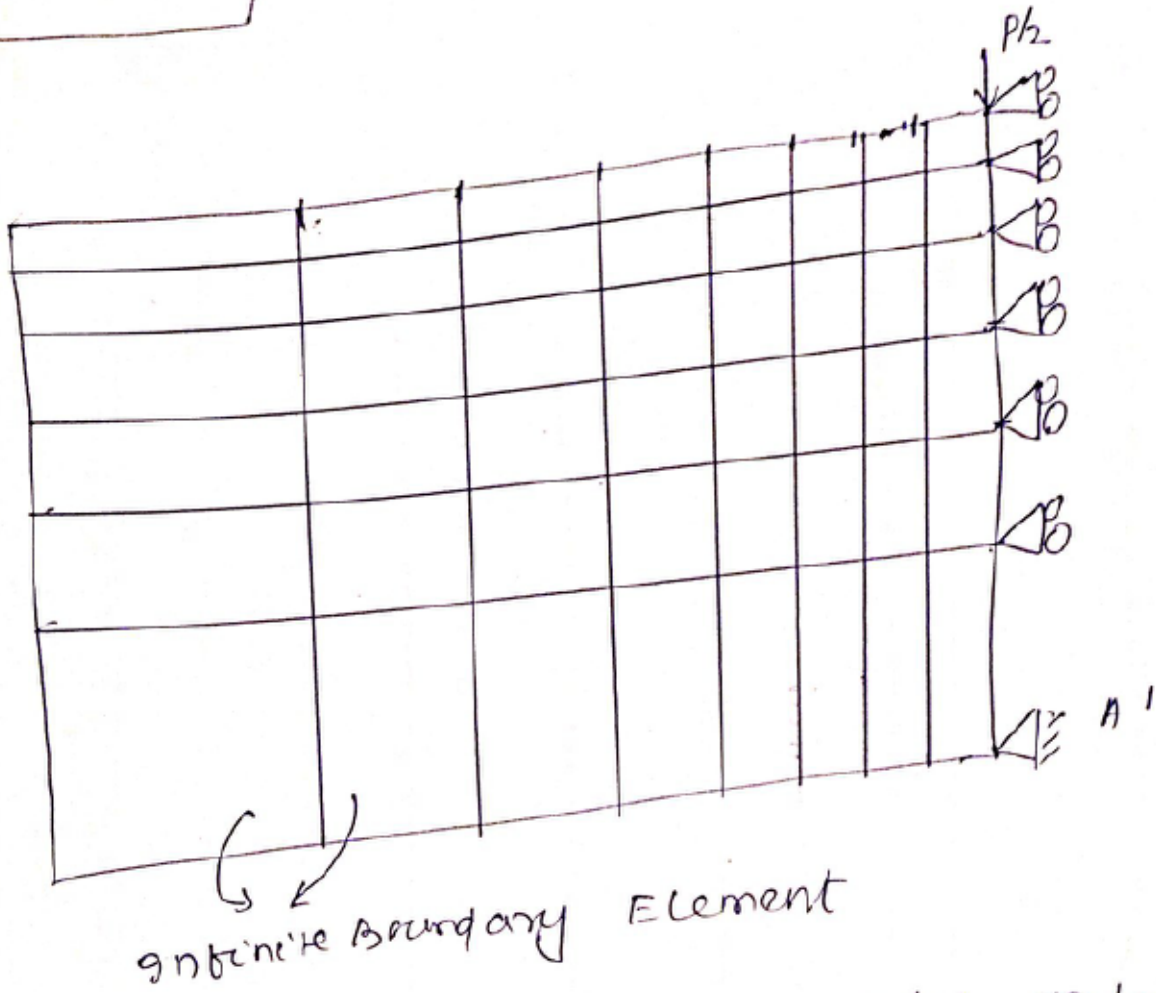
The vertical displacements of the nodes of the structure on the x axis are zero.

The horizontal displacements of the nodes of the structure on the y axis are zero.

2.10) It is possible to cut the complete structure to one half along the line of symmetry.



ONE HALF

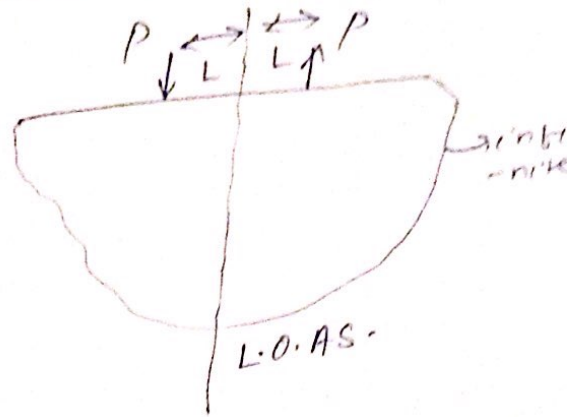


X displacements of the nodes on the line of symmetry are zero.

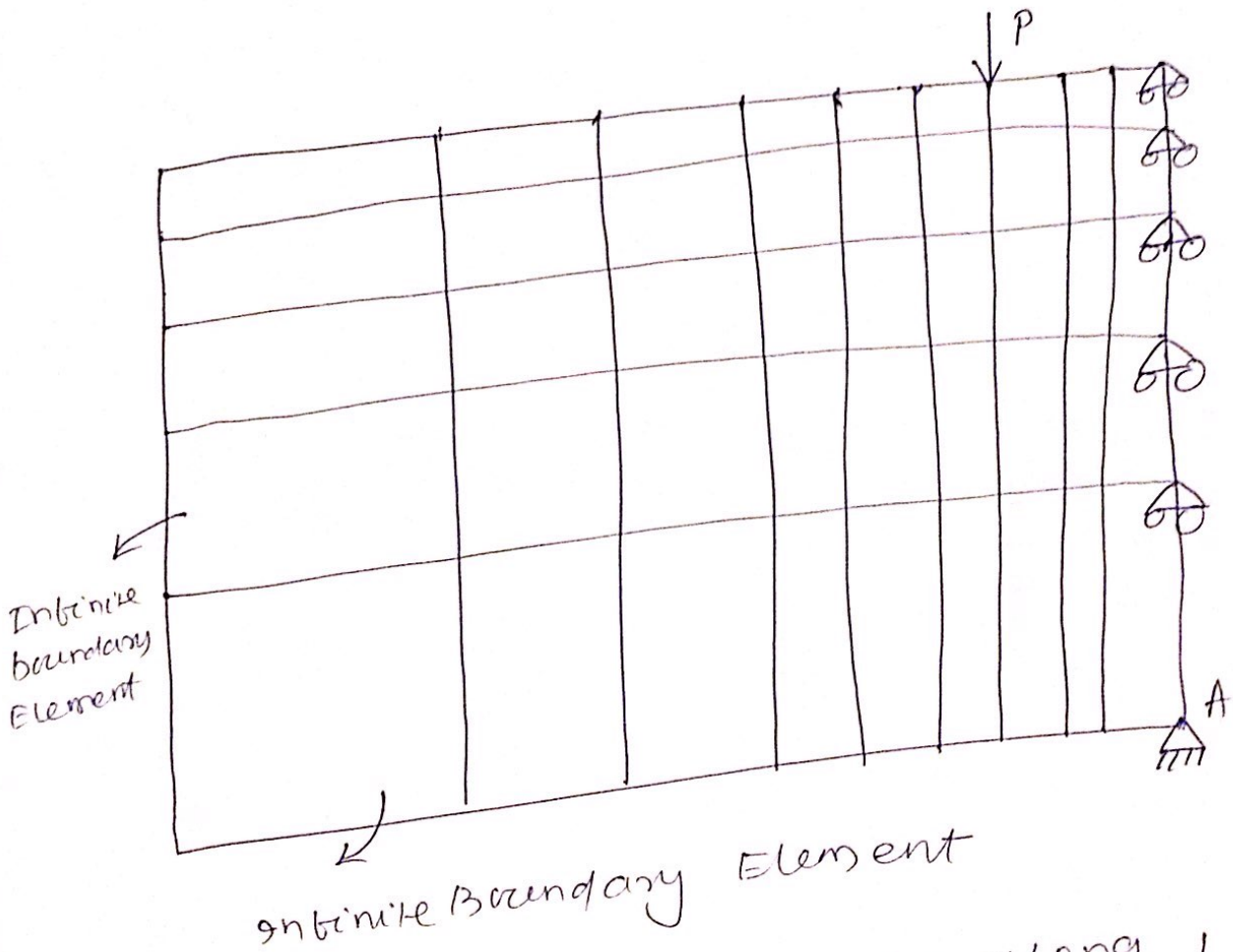
(The nodes represent infinite Element will also have very small X, Y displacements).

$A1$ is constrained in both X, Y directions to prevent the rigid body motion.

2. (b) It is possible to cut the complete structure to one half along the line of antisymmetry.

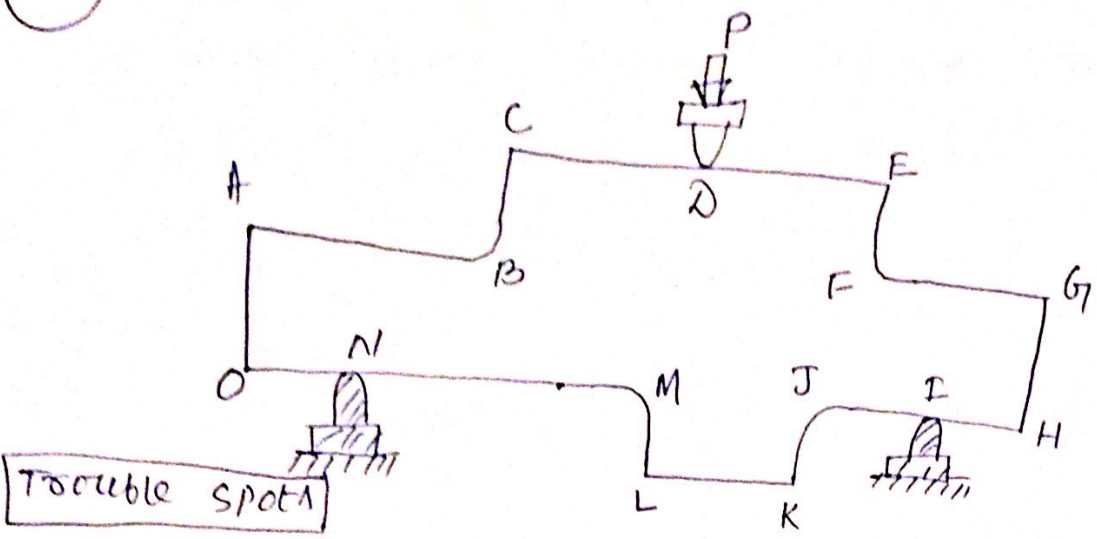


One Half



\therefore displacements of the nodes along L.O.A.S. are zero.

Also, the nodes which are at infinite will have very small x, y displacements. The constraint at point A prevent rigid body motion.



D :- vicinity of point Load

B, M, J, F :- Entrant corners or sharply curved Edges.

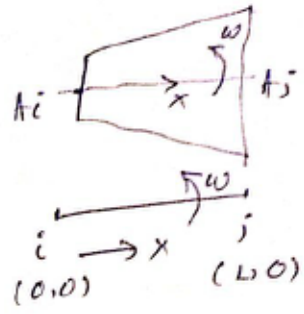
N, I :- vicinity of concentrated reactions

2.3

The length of the given tapered bar = L
 Area is interpolated as $A = A_i(1-\xi) + A_j\xi$

where $\xi = \frac{x-x_1}{L} = \frac{x-0}{L} = \frac{x}{L}$

$\Rightarrow x = \xi L$
 $\Rightarrow dx = L d\xi$



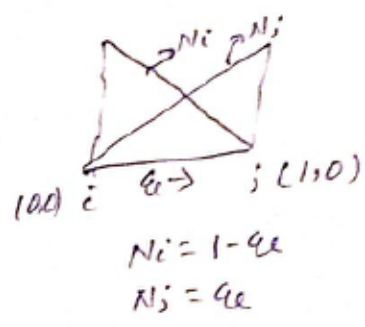
The consistent node force vector \underline{f}^e comes from the element contribution to the external work potential W :

$$W^e = \int_{x_1}^{x_2} q u dx = \int_0^1 q N^T \underline{u}^e L d\xi = (\underline{u}^e)^T \int_0^1 q \begin{bmatrix} 1-\xi \\ \xi \end{bmatrix} L d\xi$$

$\stackrel{\text{def}}{=} (\underline{u}^e)^T \underline{f}^e$

Since, \underline{u}^e is arbitrary

$$\underline{f}^e = \int_0^1 q \begin{bmatrix} 1-\xi \\ \xi \end{bmatrix} L d\xi$$



$q(x) = \gamma A \omega^2 x$
 $q(\xi) = \gamma A \omega^2 \xi L$

$$\underline{f}^e = \int_0^1 \gamma A \omega^2 L^2 \begin{bmatrix} 1-\xi^2 \\ \xi^2 \end{bmatrix} d\xi$$

$$= \gamma \omega^2 L^2 \int_0^1 [A_i(1-\xi) + A_j \xi] \begin{bmatrix} 1-\xi^2 \\ \xi^2 \end{bmatrix} d\xi$$

$$= \gamma \omega^2 L^2 \int_0^1 \begin{bmatrix} A_i(\xi - \xi^2 - \xi^2 + \xi^3) + A_j[\xi^2 - \xi^3] \\ A_i(\xi^2 - \xi^3) + A_j \xi^3 \end{bmatrix} d\xi$$

$$= \Delta \omega^2 L^2 \int_0^1 \left[\begin{array}{c} (A_i (\omega - 2\omega^2 + \omega^3) + A_j (\omega^2 - \omega^3)) \\ (A_i (\omega^2 - \omega^3) + A_j \omega^3) \end{array} \right] d\omega \quad (14)$$

$$= \Delta \omega^2 L^2 \left[\begin{array}{c} \left[A_i \left(\frac{\omega^2}{2} - \frac{2\omega^3}{3} + \frac{\omega^4}{4} \right) + A_j \left(\frac{\omega^3}{3} - \frac{\omega^4}{4} \right) \right]_0^1 \\ \left[A_i \left(\frac{\omega^3}{3} - \frac{\omega^4}{4} \right) + A_j \frac{\omega^4}{4} \right]_0^1 \end{array} \right]$$

$$= \Delta \omega^2 L^2 \left[\begin{array}{c} A_i \left(\frac{1}{2} - \frac{2}{3} + \frac{1}{4} \right) + A_j \left(\frac{1}{3} - \frac{1}{4} \right) \\ A_i \left(\frac{1}{3} - \frac{1}{4} \right) + A_j \frac{1}{4} \end{array} \right]$$

$$\Rightarrow \underline{b}^{(e)} = \Delta \omega^2 L^2 \left[\begin{array}{c} \frac{A_i}{12} + \frac{A_j}{12} \\ \frac{A_i}{12} + \frac{A_j}{4} \end{array} \right]$$

$$\Rightarrow \left[\begin{array}{c} b_i^e \\ b_j^e \end{array} \right] = \Delta \omega^2 L^2 \left[\begin{array}{c} \frac{A_i + A_j}{12} \\ \frac{A_i + 3A_j}{12} \end{array} \right] \quad (AM)$$

For prismatic Bar ($A_i = A_j$); $A_i = A_j = A$ (say)

$$\left[\begin{array}{c} b_i^e \\ b_j^e \end{array} \right] = \Delta \omega^2 L^2 \left[\begin{array}{c} \frac{A}{6} \\ \frac{A}{3} \end{array} \right] = \left[\begin{array}{c} \frac{\Delta \omega^2 L^2 A}{6} \\ \frac{\Delta \omega^2 L^2 A}{3} \end{array} \right] \quad (AM)$$