

# Assignment 4

[structure of Revolution]

1

→ Assignment 4.1 [Axisymmetric triangle].

QI) Compute the entries of  $k^e$

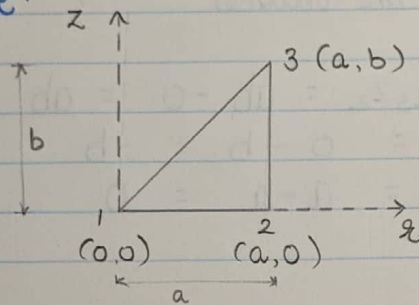
→ Solution:

Step ① Element type:

Pt 1 ( $x_1, z_1$ ) = (0, 0)

Pt 2 ( $x_2, z_2$ ) = (a, 0)

Pt 3 ( $x_3, z_3$ ) = (a, b)



2 DOF per node  
3 node - 6 DOF

Step ② Displacement function:

$$\begin{Bmatrix} U(x,z) \\ w(x,z) \end{Bmatrix} = \begin{bmatrix} N_1 & 0 & N_2 & 0 & N_3 & 0 \\ 0 & N_1 & 0 & N_2 & 0 & N_3 \end{bmatrix} \begin{Bmatrix} U_1 \\ w_1 \\ U_2 \\ w_2 \\ U_3 \\ w_3 \end{Bmatrix}$$

where,

$$N_1 = \frac{1}{2A} (\alpha_1 + \beta_1 x + \gamma_1 z)$$

$$N_2 = \frac{1}{2A} (\alpha_2 + \beta_2 x + \gamma_2 z)$$

$$N_3 = \frac{1}{2A} (\alpha_3 + \beta_3 x + \gamma_3 z)$$

step ③ strain

$$[\epsilon] = \begin{Bmatrix} \epsilon_{xx} \\ \epsilon_{zz} \\ \epsilon_{\theta\theta} \\ \gamma_{xz} \end{Bmatrix} = \begin{bmatrix} \frac{\partial N_1}{\partial x} & 0 & \frac{\partial N_2}{\partial x} & 0 & \frac{\partial N_3}{\partial x} & 0 \\ 0 & \frac{\partial N_1}{\partial z} & 0 & \frac{\partial N_2}{\partial z} & 0 & \frac{\partial N_3}{\partial z} \\ 2AN_1/x & 0 & 2AN_2/x & 0 & 2AN_3/x & 0 \\ \frac{\partial N_1}{\partial z} & \frac{\partial N_1}{\partial x} & \frac{\partial N_2}{\partial z} & \frac{\partial N_2}{\partial x} & \frac{\partial N_3}{\partial z} & \frac{\partial N_3}{\partial x} \end{bmatrix} \begin{Bmatrix} U_1 \\ w_1 \\ U_2 \\ w_2 \\ U_3 \\ w_3 \end{Bmatrix}$$

Taking derivative of shape function & substituting the values,

$$[\epsilon] = \frac{1}{2A} \begin{bmatrix} \beta_1 & 0 & \beta_2 & 0 & \beta_3 & 0 \\ 0 & \gamma_1 & 0 & \gamma_2 & 0 & \gamma_3 \\ \frac{\alpha_1 + \beta_1 x + \gamma_1 z}{x} & 0 & \frac{\alpha_2 + \beta_2 x + \gamma_2 z}{x} & 0 & \frac{\alpha_3 + \beta_3 x + \gamma_3 z}{x} & 0 \\ \gamma_1 & \beta_1 & \gamma_2 & \beta_2 & \gamma_3 & \beta_3 \end{bmatrix} \begin{bmatrix} u_1 \\ w_1 \\ u_2 \\ w_2 \\ u_3 \\ w_3 \end{bmatrix}$$

Now calculating the values -

At Node 1

$$\alpha_1 = x_2 z_3 - x_3 z_2 = ab - 0 = ab$$

$$\beta_1 = z_2 - z_3 = 0 - b = -b$$

$$\gamma_1 = x_3 - x_2 = a - a = 0$$

Note 2:

$$\alpha_2 = x_3 x_1 - x_1 z_3 = 0 - 0 = 0$$

$$\beta_2 = z_3 - z_1 = b - 0 = b$$

$$\gamma_2 = x_1 - x_3 = 0 - a = -a$$

Note 3:

$$\alpha_3 = x_1 z_2 - x_2 z_1 = 0 - 0 = 0$$

$$\beta_3 = z_1 - z_2 = 0 - 0 = 0$$

$$\gamma_3 = x_2 - x_1 = a - 0 = a$$

Area:

$$2A = \frac{1}{2} \times \text{base} \times \text{height} = \frac{1}{2} \times a \times b = \frac{ab}{2}$$

or,

$$2A = \det \begin{vmatrix} 1 & 1 & 1 \\ x_1 & x_2 & x_3 \\ z_1 & z_2 & z_3 \end{vmatrix} = \det \begin{vmatrix} 1 & 1 & 1 \\ 0 & a & a \\ 0 & 0 & b \end{vmatrix}$$

$$\text{ie. } A = \frac{ab}{2}$$

Step ④ Stiffness Matrix -

$$[K] = 2\pi \iint_A [\bar{B}]^T [\bar{D}] [\bar{B}] \rho \, dx \, dz$$

To evaluate [B] by centroid point  $(\bar{x}, \bar{z})$  of the element.

ie.  $[B(\bar{x}, \bar{z})] = [\bar{B}]$

& Centroid formula -

$$x = \bar{x} = \frac{x_1 + x_2 + x_3}{3} = \frac{0 + a + a}{3} = \frac{2a}{3}$$

$$z = \bar{z} = \frac{z_1 + z_2 + z_3}{3} = \frac{0 + 0 + b}{3} = \frac{b}{3}$$

& Stiffness matrix (consideration)

The triangular subdivisions are consistent with the final stress distribution, then the stiffness matrix is

$$[K] = 2\pi \bar{x} A [\bar{B}]^T [\bar{D}] [\bar{B}]$$

Calculating the step ⑤ values of substitution -

$$\frac{\alpha_1 + \beta_1 \bar{x} + \gamma_1 \bar{z}}{\bar{x}} = \frac{3}{2a} (ab + (-b) \frac{2a}{3} + 0) = \frac{b}{2}$$

$$\frac{\alpha_2 + \beta_2 \bar{x} + \gamma_2 \bar{z}}{\bar{x}} = \frac{3}{2a} (0 + \frac{2ab}{3} + \frac{-ab}{3}) = \frac{b}{2}$$

$$\frac{\alpha_3 + \beta_3 \bar{x} + \gamma_3 \bar{z}}{\bar{x}} = \frac{3}{2a} (0 + 0 + \frac{ab}{3}) = \frac{b}{2}$$

B matrix -

$$[B] = \frac{1}{2A} \begin{bmatrix} -b & 0 & b & 0 & 0 & 0 \\ 0 & 0 & 0 & -a & 0 & a \\ \frac{b}{2} & 0 & \frac{b}{2} & 0 & \frac{b}{2} & 0 \\ 0 & -b & -a & b & a & 0 \end{bmatrix}$$

$B_1$ 
 $B_2$ 
 $B_3$

$$B^T E B = \frac{E}{4A^2} \begin{bmatrix} -b & 0 & b/2 & 0 \\ 0 & 0 & 0 & -b \\ b & 0 & b/2 & -a \\ 0 & -a & 0 & b \\ 0 & 0 & b/2 & a \\ 0 & a & 0 & 0 \end{bmatrix} \begin{bmatrix} 1 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 \\ 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 1/2 \end{bmatrix} \begin{bmatrix} -b & 0 & b & 0 & 0 & 0 \\ 0 & 0 & 0 & -a & 0 & a \\ b/2 & 0 & b/2 & 0 & b/2 & 0 \\ 0 & -b & -a & b & a & 0 \end{bmatrix}$$

$$B^T E B = \frac{E}{(ab)^2} \begin{bmatrix} \frac{5b^2}{4} & 0 & -\frac{3b^2}{4} & 0 & \frac{b^2}{4} & 0 \\ 0 & \frac{b^2}{2} & \frac{ab}{2} & -\frac{b^2}{2} & -\frac{ab}{2} & 0 \\ -\frac{3b^2}{4} & \frac{ab}{2} & \frac{5b^2+a^2}{4} & -\frac{ab}{2} & \frac{b^2}{4} - \frac{a^2}{2} & 0 \\ 0 & -\frac{b^2}{2} & -\frac{ab}{2} & \frac{a^2+b^2}{2} & \frac{ab}{2} & -a^2 \\ \frac{b^2}{4} & -\frac{ab}{2} & \frac{b^2}{4} - \frac{a^2}{2} & \frac{ab}{2} & \frac{b^2}{4} + \frac{a^2}{2} & 0 \\ 0 & 0 & 0 & -a^2 & 0 & a^2 \end{bmatrix}$$

The Element stiffness matrix [K]:

$$[K] = \frac{2\pi E}{3b} \begin{bmatrix} \frac{5b^2}{4} & 0 & -\frac{3b^2}{4} & 0 & \frac{b^2}{4} & 0 \\ 0 & \frac{b^2}{2} & \frac{ab}{2} & -\frac{b^2}{2} & -\frac{ab}{2} & 0 \\ -\frac{3b^2}{4} & \frac{ab}{2} & \frac{5b^2+a^2}{4} & -\frac{ab}{2} & \frac{b^2}{4} - \frac{a^2}{2} & 0 \\ 0 & -\frac{b^2}{2} & -\frac{ab}{2} & \frac{a^2+b^2}{2} & \frac{ab}{2} & -a^2 \\ \frac{b^2}{4} & -\frac{ab}{2} & \frac{b^2}{4} - \frac{a^2}{2} & \frac{ab}{2} & \frac{b^2}{4} + \frac{a^2}{2} & 0 \\ 0 & 0 & 0 & -a^2 & 0 & a^2 \end{bmatrix}$$

Simplifying,

$$[K] = 2\pi E \begin{bmatrix} \frac{5b}{12} & 0 & -\frac{b}{4} & 0 & \frac{b^2}{12} & 0 \\ 0 & \frac{b}{6} & \frac{a}{6} & -\frac{b}{6} & -\frac{a}{6} & 0 \\ -\frac{b}{4} & \frac{a}{6} & \frac{5b}{12} + \frac{a^2}{6b} & -\frac{a}{6} & \frac{b}{12} - \frac{a^2}{6b} & 0 \\ 0 & -\frac{b}{6} & -\frac{a}{6} & \frac{a^2}{3b} + \frac{b}{6} & \frac{a}{6} & -\frac{a^2}{3b} \\ \frac{b}{12} & -\frac{a}{6} & \frac{b}{12} - \frac{a^2}{6b} & \frac{a}{6} & \frac{b}{12} + \frac{a^2}{6b} & 0 \\ 0 & 0 & 0 & -\frac{a^2}{3b} & 0 & \frac{a^2}{3b} \end{bmatrix}$$

Q II) To show row (2), (4) & (6) vanishes and row (1), (3) & (5) not vanishes. Why?

(i) For row (2), (4) & (6)

On keen observing the stiffness matrix, the addition of row (2), (4) & (6) is zero (vanishes).

$$\text{Row (2) + Row (4) + Row (6)} = \begin{bmatrix} 0 & \frac{b}{6} & \frac{a}{6} & -\frac{b}{6} & -\frac{a}{6} & 0 \\ 0 & -\frac{b}{6} & -\frac{a}{6} & \frac{a^2}{3b} + \frac{b}{6} & \frac{a}{6} & -\frac{a^2}{3b} \\ \frac{a}{6} & -\frac{a^2}{3b} & 0 & 0 & 0 & \frac{a^2}{3b} \end{bmatrix} + \begin{bmatrix} 0 & 0 & 0 & -\frac{a^2}{3b} & 0 & \frac{a^2}{3b} \end{bmatrix}$$

$$\text{Row (2) + Row (4) + Row (6)} = [0 \ 0 \ 0 \ 0 \ 0 \ 0]$$

The rigid body motion is achieved in z direction. We get zero internal resistance forces to the prescribed motion, which delineates the rigid body motion in z direction.

$$\text{ie. } [U^e] = [U_1 \ w_1 \ u_2 \ w_2 \ u_3 \ w_3]^T = [0 \ 1 \ 0 \ 1 \ 0 \ 1]^T$$

(ii) For row (1), (3) & (5)

On keen observing the stiffness matrix, the addition of row (1), (3) & (5) is not zero.

$$\text{Row}(1) + \text{Row}(3) + \text{Row}(5) = \begin{bmatrix} \frac{5b^2}{12} & 0 & -\frac{b}{4} & 0 & \frac{b}{12} & 0 \end{bmatrix} + \begin{bmatrix} -\frac{b}{4} & \frac{a}{6} & \frac{a^2}{6b} + \frac{5b}{12} & -\frac{a}{6} & 0 & 0 \end{bmatrix} + \begin{bmatrix} \frac{b}{12} & -\frac{a^2}{6b} & 0 & \frac{b}{12} & -\frac{a}{6} & \frac{a}{6} & \frac{b}{12} + \frac{a^2}{6b} & 0 \end{bmatrix}$$

$$\text{Row}(1) + \text{Row}(3) + \text{Row}(5) \neq [0 \ 0 \ 0 \ 0 \ 0 \ 0]$$

So, the rigid body motion is not achieved in x direction.

Q III) The consistent force vector  $f^e$ .

$$[b] = [0 \ -q]^T$$

The body force ( $F_b$ ) -

$$[F_b] = 2\pi \iint_A [N]^T \begin{Bmatrix} R_b \\ z_b \end{Bmatrix} r \, dr \, dz$$

$[F_b]$  is also evaluated by centroid of the element and  $\bar{R}_b$  is radially directed body force.

$$\therefore [F_b] = \frac{2\pi \bar{r} A}{3} \begin{Bmatrix} \bar{R}_b \\ \bar{z}_b \end{Bmatrix}$$

$$\text{where } [N]^T = \begin{bmatrix} N_1^e & 0 \\ 0 & N_i^e \end{bmatrix}$$

ie,

$$[F_b] = \frac{2\pi \bar{r} A}{3} \begin{Bmatrix} R_{b1} \\ R_{b1} \\ R_{b2} \\ Z_{b2} \\ R_{b3} \\ Z_{b3} \end{Bmatrix}$$

Substituting the values,

$$b = [0 \ 1 \ 0]^T \quad b = [r_b \ z_b]^T = [0 \ -\rho g]^T$$

$$\bar{\epsilon} = \frac{2a}{3}, \quad A = \frac{ab}{2}$$

$$[F_b] = \frac{2\pi}{3} \times \frac{2a}{3} \times \frac{ab}{2} \begin{bmatrix} 0 \\ -\rho g \\ 0 \\ -\rho g \\ 0 \\ -\rho g \end{bmatrix}$$

$$[F_b] = 2\pi \left[ 0 \quad -\frac{\rho g a^2 b}{9} \quad 0 \quad -\frac{\rho g a^2 b}{9} \quad 0 \quad -\frac{\rho g a^2 b}{9} \right]^T$$

$$[F_b] = -\frac{2\pi \rho g a^2 b}{9} [0 \ 1 \ 0 \ 1 \ 0 \ 1]^T$$

— x — x — x —

Reference: - 'A First Course in the Finite Element Method' by Daryl L. Logan

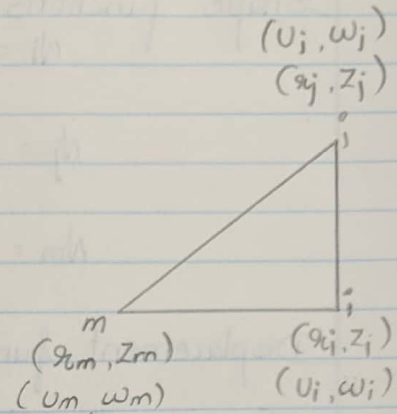
Ref 1

⇒ Axisymmetric Elements -  
Derivation of Stiffness Matrix:

Step (1) Element Type -

An axisymmetric solid, along with a triangular element.

The element has 3 nodes with 2 degrees of freedom per node.



Step (2) Displacement functions

$$U(r, z) = a_1 + a_2 r + a_3 z$$

$$w(r, z) = a_4 + a_5 r + a_6 z$$

where, the total number of  $a_i$ 's i.e. displacement functions is the same as the total number of degrees of freedom for the element.

Displacement Matrix -

$$\begin{bmatrix} U \\ w \end{bmatrix} = \begin{bmatrix} a_1 + a_2 r + a_3 z \\ a_4 + a_5 r + a_6 z \end{bmatrix} = \begin{bmatrix} 1 & r & z & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & r & z \end{bmatrix} \begin{bmatrix} a_1 & a_2 & a_3 & a_4 & a_5 & a_6 \end{bmatrix}^T$$

ie,

$$\begin{bmatrix} a_1 \\ a_2 \\ a_3 \end{bmatrix} = \begin{bmatrix} 1 & r_i & z_i \\ 1 & r_j & z_j \\ 1 & r_m & z_m \end{bmatrix}^{-1} \begin{bmatrix} U_i \\ U_j \\ U_m \end{bmatrix} \quad \& \quad \begin{bmatrix} a_4 \\ a_5 \\ a_6 \end{bmatrix} = \begin{bmatrix} 1 & r_i & z_i \\ 1 & r_j & z_j \\ 1 & r_m & z_m \end{bmatrix}^{-1} \begin{bmatrix} w_i \\ w_j \\ w_m \end{bmatrix}$$

After performing inversion operation

$$\begin{bmatrix} a_1 \\ a_2 \\ a_3 \end{bmatrix} = \frac{1}{2A} \begin{bmatrix} \alpha_i & \alpha_j & \alpha_m \\ \beta_i & \beta_j & \beta_m \\ \gamma_i & \gamma_j & \gamma_m \end{bmatrix} \begin{bmatrix} U_i \\ U_j \\ U_m \end{bmatrix} \quad \& \quad \begin{bmatrix} a_4 \\ a_5 \\ a_6 \end{bmatrix} = \frac{1}{2A} \begin{bmatrix} \alpha_i & \alpha_j & \alpha_m \\ \beta_i & \beta_j & \beta_m \\ \gamma_i & \gamma_j & \gamma_m \end{bmatrix} \begin{bmatrix} w_i \\ w_j \\ w_m \end{bmatrix}$$

where,

$$\begin{aligned} \alpha_i &= r_j z_m - r_m z_j & \beta_i &= z_j - z_m & \gamma_i &= r_m - r_j \\ \alpha_j &= r_m z_i - r_i z_m & \beta_j &= z_m - z_i & \gamma_j &= r_i - r_m \\ \alpha_m &= r_i z_j - r_j z_i & \beta_m &= z_i - z_j & \gamma_m &= r_j - r_i \end{aligned}$$



Shape functions -

$$N_i = \frac{1}{2A} (\alpha_i + \beta_i x + \gamma_i z)$$

$$N_j = \frac{1}{2A} (\alpha_j + \beta_j x + \gamma_j z)$$

$$N_m = \frac{1}{2A} (\alpha_m + \beta_m x + \gamma_m z)$$

Displacement function -

$$\psi = \begin{bmatrix} U(x,z) \\ w(x,z) \end{bmatrix} = \begin{bmatrix} N_i & 0 & N_j & 0 & N_m & 0 \\ 0 & N_i & 0 & N_j & 0 & N_m \end{bmatrix} \begin{bmatrix} U_i \\ w_i \\ U_j \\ w_j \\ U_m \\ w_m \end{bmatrix}$$

ie,  $\{\psi\} = [N][d]$

step(3) Strain & stress :

$$[\epsilon] = \begin{bmatrix} \epsilon_{xx} \\ \epsilon_{zz} \\ \epsilon_{\theta\theta} \\ \gamma_{xz} \end{bmatrix} = \begin{bmatrix} 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \\ \frac{1}{x} & 1 & \frac{z}{x} & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 1 & 0 \end{bmatrix} \begin{bmatrix} a_1 \\ a_2 \\ a_3 \\ a_4 \\ a_5 \\ a_6 \end{bmatrix}$$

on simplifying -

$$[\epsilon] = \frac{1}{2A} \begin{bmatrix} \beta_i & 0 & \beta_j & 0 & \beta_m & 0 \\ 0 & \gamma_i & 0 & \gamma_j & 0 & \gamma_m \\ \frac{(\alpha_i + \beta_i x + \gamma_i z)}{x} & 0 & \frac{(\alpha_j + \beta_j x + \gamma_j z)}{x} & 0 & \frac{(\alpha_m + \beta_m x + \gamma_m z)}{x} & 0 \\ \gamma_i & \beta_i & \gamma_j & \beta_j & \gamma_m & \beta_m \end{bmatrix} \begin{bmatrix} U_i \\ w_i \\ U_j \\ w_j \\ U_m \\ w_m \end{bmatrix}$$

ie,

$$[\epsilon] = [\bar{B}][d]$$

$$\dots [\bar{B}] = [\bar{B}_i \quad \bar{B}_j \quad \bar{B}_m]$$

where,

$$[B_i] = \frac{1}{2A} \begin{bmatrix} \beta_i & 0 \\ 0 & \gamma_i \\ \frac{\alpha_i + \beta_i x + \gamma_i z}{r} & 0 \\ \gamma_i & \beta_i \end{bmatrix}$$

stress -

$$[\sigma] = [D][B][d]$$

Step (4) Element stiffness Matrix.

The stiffness matrix (K) :

$$[K] = \iiint_V [B]^T [D] [B] dV$$

$$[K] = 2\pi \iint_A [B]^T [D] [B] r dr dz$$

As [B] is function of  $r, z$ . Also, [K] is function of  $r$  &  $z$ .

To evaluate K, we can use many methods such as

- (i) Numerical Integration.
- (ii) Explicit multiplication & term by term multiplication integration.
- (iii) Evaluate B by centroid point  $(\bar{x}, \bar{z})$  of the element.

ie,

By centroid -

$$[K] = 2\pi \bar{x} A [B]^T [D] [B]$$

— x — x — x —